



Kentucky Concrete Pavement Forensic Analysis



**Southeastern States Pavement Management
and Design Conference**

May 9, 2006

**Kentucky Transportation Cabinet
Division of Highway Design**

Kentucky Interstate System

- Original Construction (1960-1980)

 - Concrete = 70%

 - Asphalt = 30%

- Current (Rehabilitated 1985-Present)

 - Concrete = 7%

 - Asphalt = 93%

1960's Era Pavements

- Jointed Plain Concrete Pavement
 - 50-foot joint spacing
- Average Age to Rehabilitation = 25.5 years
- Major Distresses
 - Joint deterioration
 - Mid-panel cracking

1980's Era Pavements

- Jointed Plain Concrete Pavement
 - 12,13,15,17 random joint spacing
- Average Age to Rehabilitation = 20 years
- Major Distresses
 - Joint Faulting
 - Tie-bar and Dowel-bar deterioration

Concrete Pavement Forensic Analysis

2 case studies

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Overview

■ Case study #1

- Using GPR to determine why a concrete pavement (PCCP) has been settling on I-265 in Louisville, Kentucky and how it has changed the project design

■ Case study #2

- Different strategies for collecting concrete pavement distress information for rehabilitation design purposes—Project I-65 Central Kentucky

■ Questions

Case Study: #1

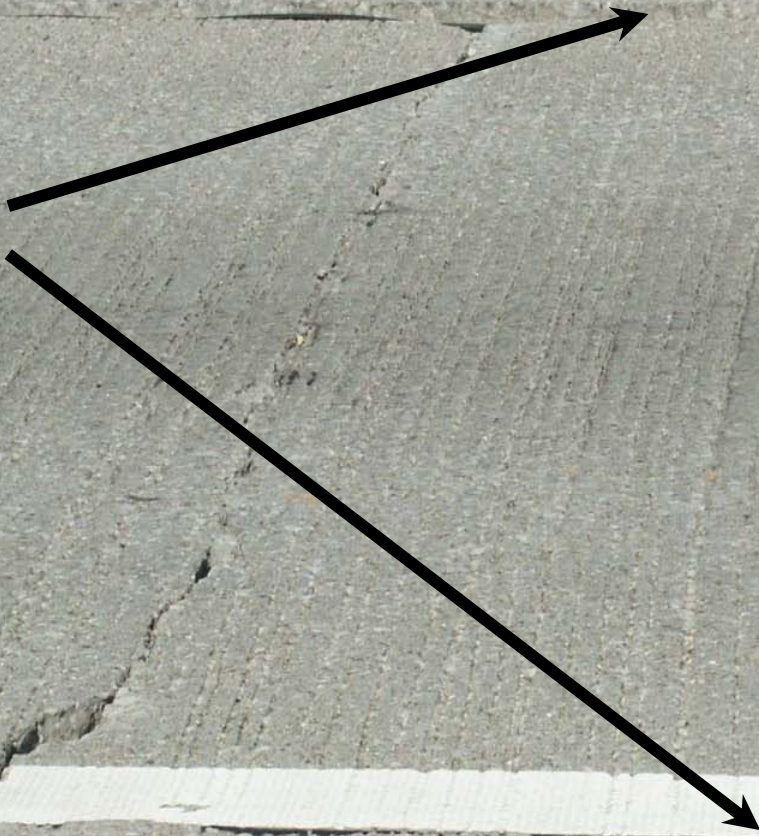
Using GPR to identify why a PCCP pavement has been settling

■ Project background

- 3-mile long project on I-265 in Louisville, KY. Mileposts 15 to 18
- 10 inch concrete pavement that was constructed in 1987
- Right driving lane has differentially settled approximately 1 to 2 inches from the shoulder and the left driving lanes
- Urgency for inspection: 2003, one motorcycle fatality had occurred due to the pavement settlement

Station 1687+22 Eastbound

1.25" Lane Drop



What did we use and how did we use it?



Ground Penetrating Radar

Equipment

900 MHz. antenna
Approx. depth 3 ft.

Data collection location

1 pass per lane
CWP

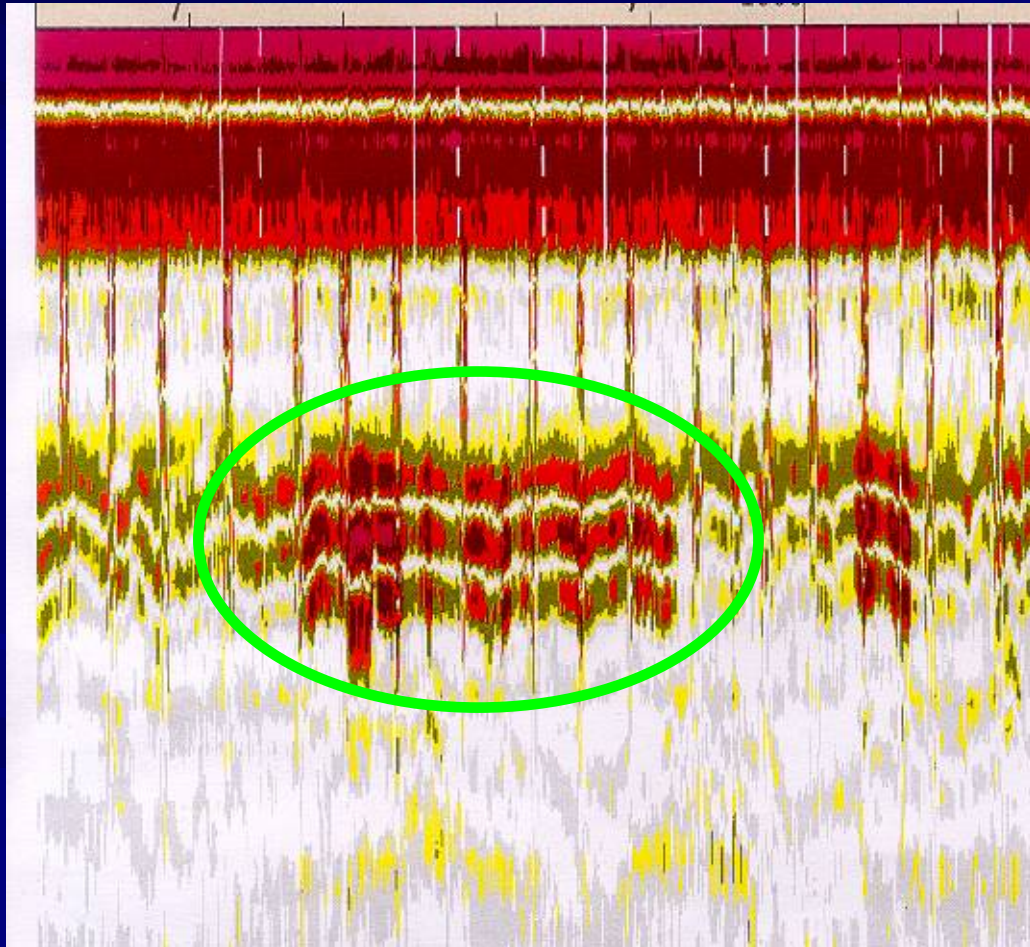
Data collection density

3 scans per foot

Data collection speed

20 m.p.h. (3 hrs.)

First: used GPR to locate different degrees of saturated sub-base beneath PCCP

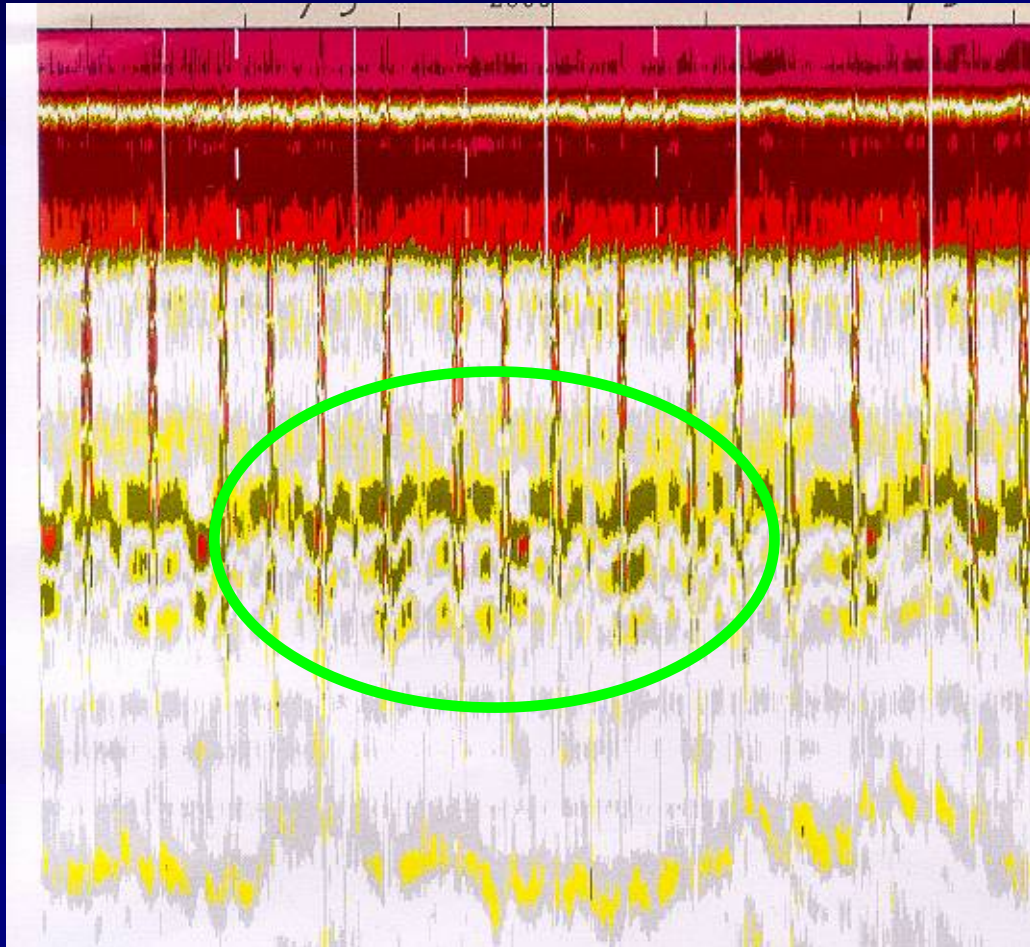


Degree of saturation scale

severe
moderate
minimum



Moderate water beneath PCCP

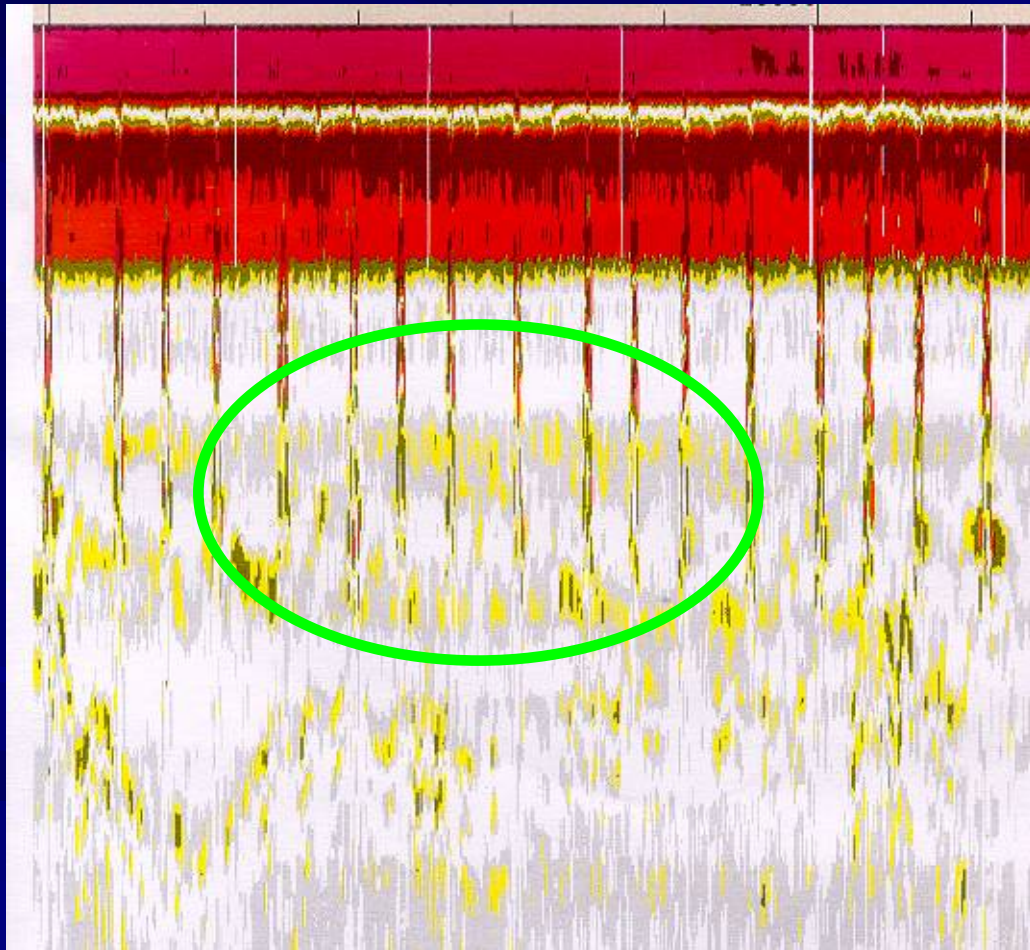


Degree of saturation scale

severe
moderate
minimum



Minimum water beneath PCCP



Degree of saturation scale

severe
moderate
minimum



Second: used GPR to locate tie bars between lanes



1.25" Lane Drop



Driving
Lane
Side

Shoulder
Side

CE 081

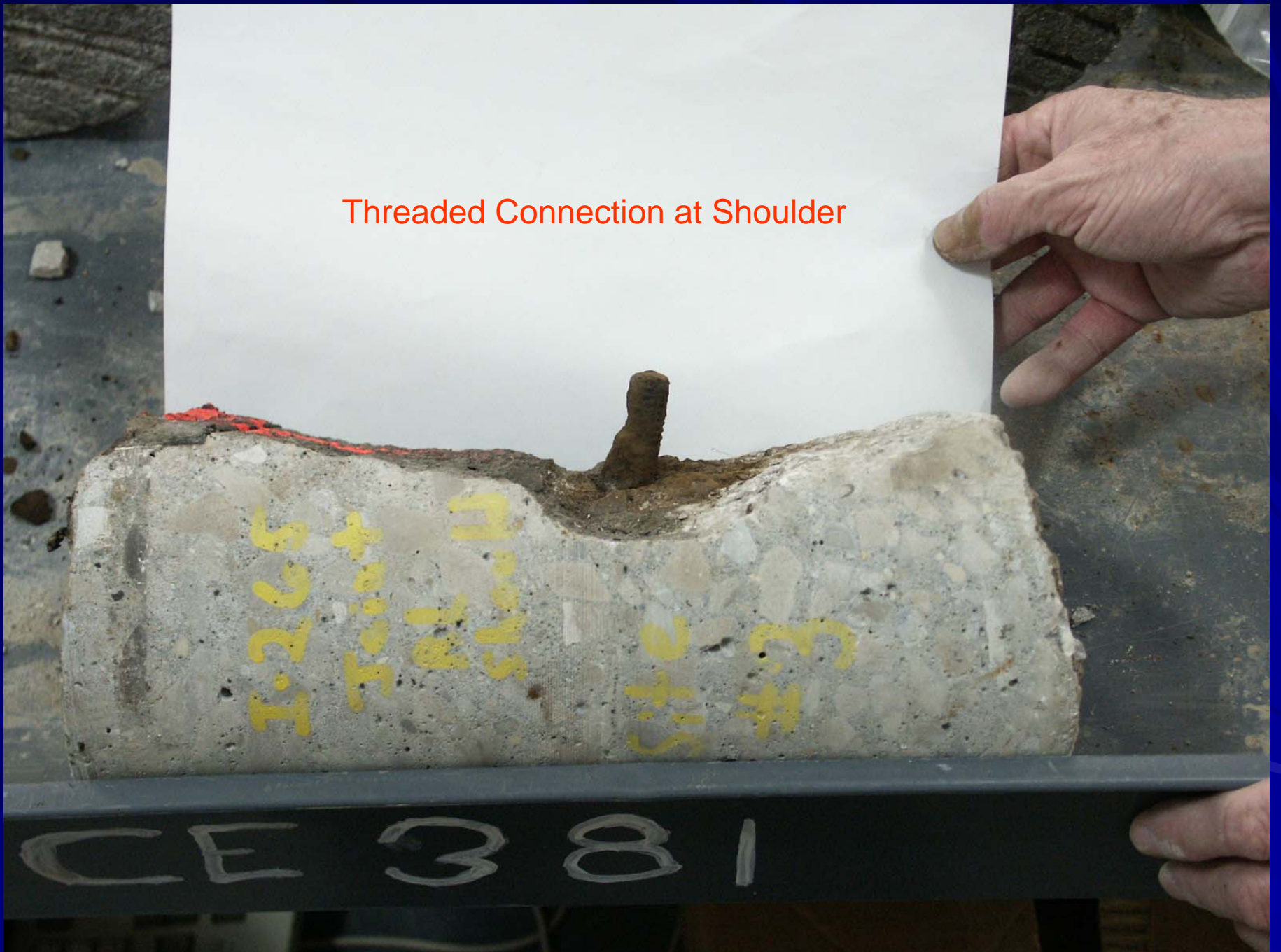
Driving Lane Side



Shoulder Side



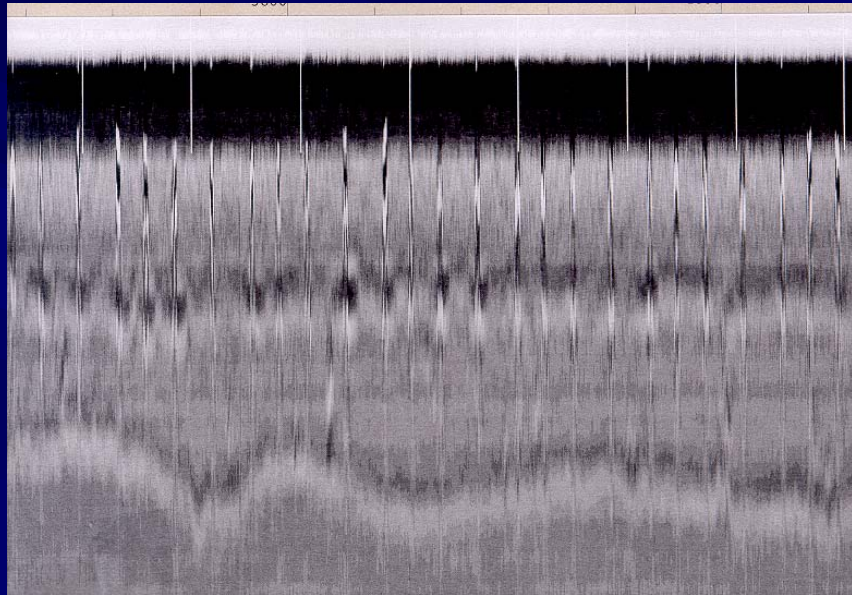
Threaded Connection at Shoulder



Third: used GPR to locate dowel bars at the transverse joint

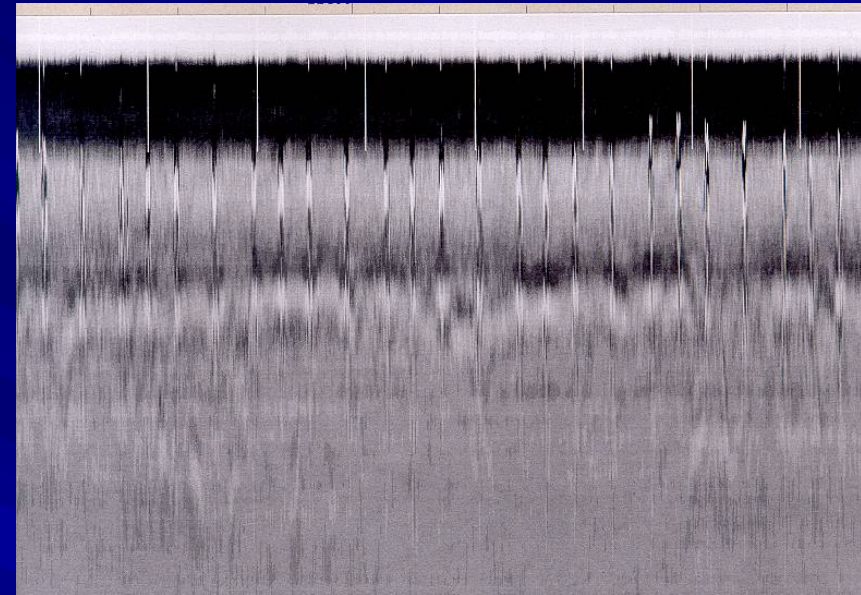


Fourth: GPR was used to find clay layer between D.G.A. and apparent rock roadbed



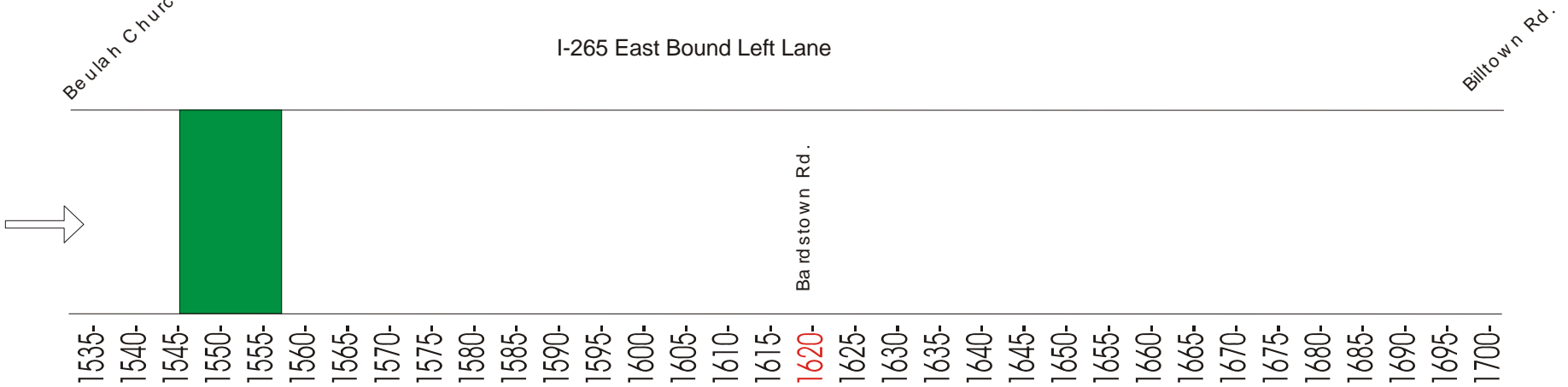
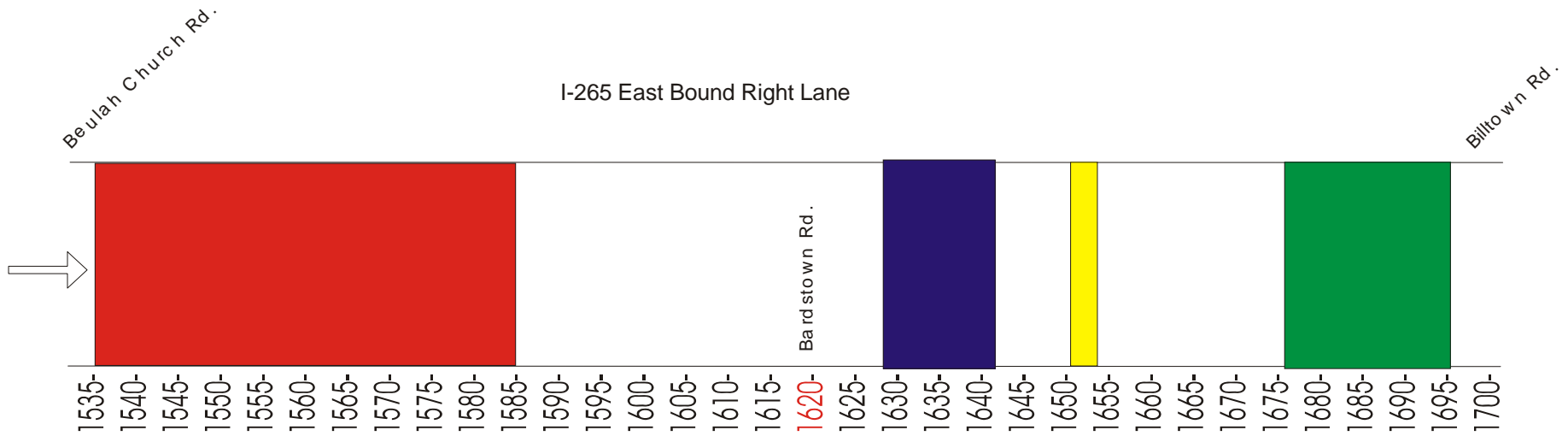
Core Information

Station 1582+43
Approx. 2" between lane faulting
PCCP: 9.75" - 10.25"
DGA: 5.75" - 6.25"
Clay soil: 8" - 10"



Core Information

Station 1699+73
No between lane faulting
PCCP: 10.00" - 10.25"
DGA: 4.75" - 5.25"
Apparent rock roadbed:



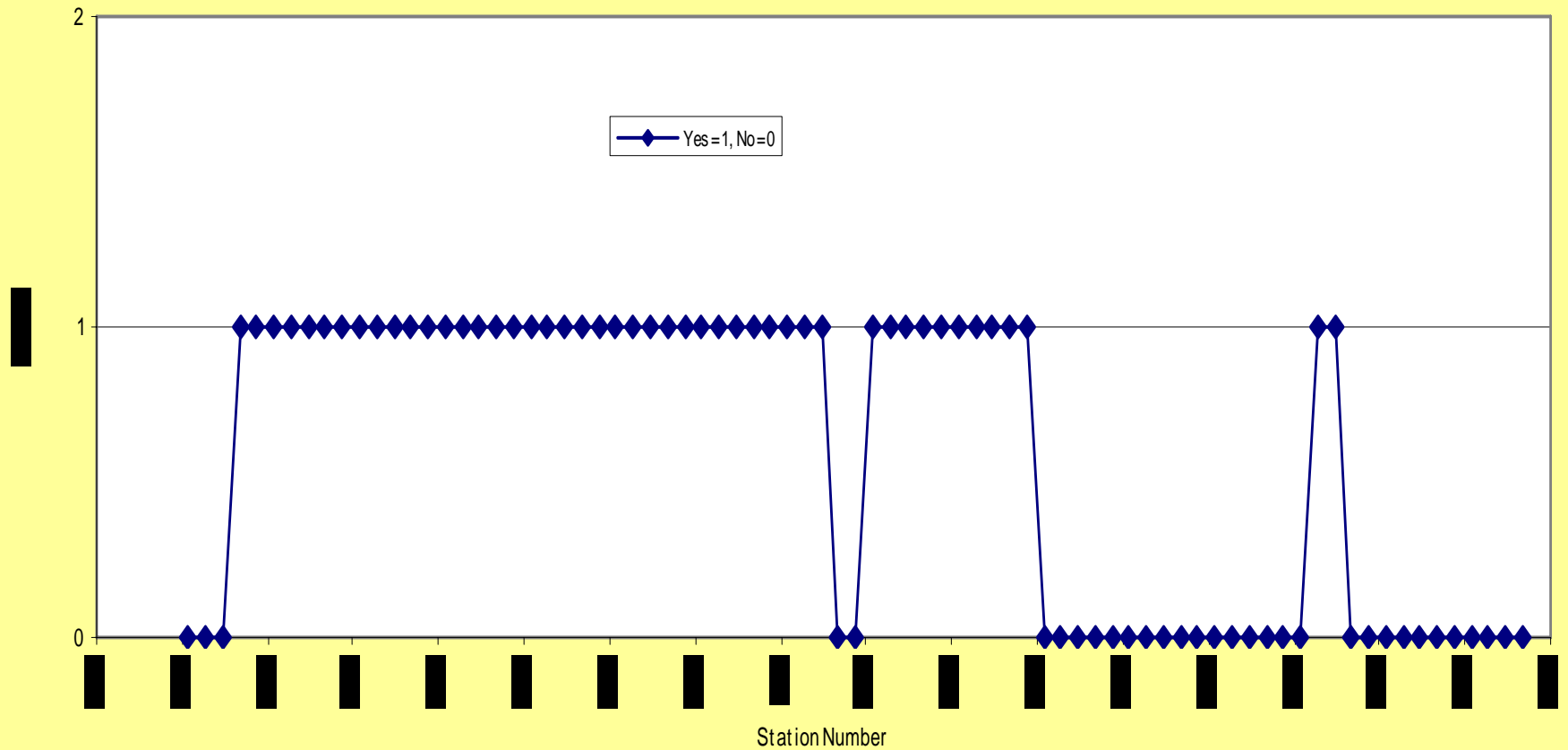
Red: 1" or more settlement, less than 50% load transfer, & moderate to severe water

Dark Blue: 1" settlement and severe water

Yellow: 1" settlement (possible slab jacking)

Green: Less than 50% load transfer, some settlement and/or water

Presence of Clay layer (yes/no)



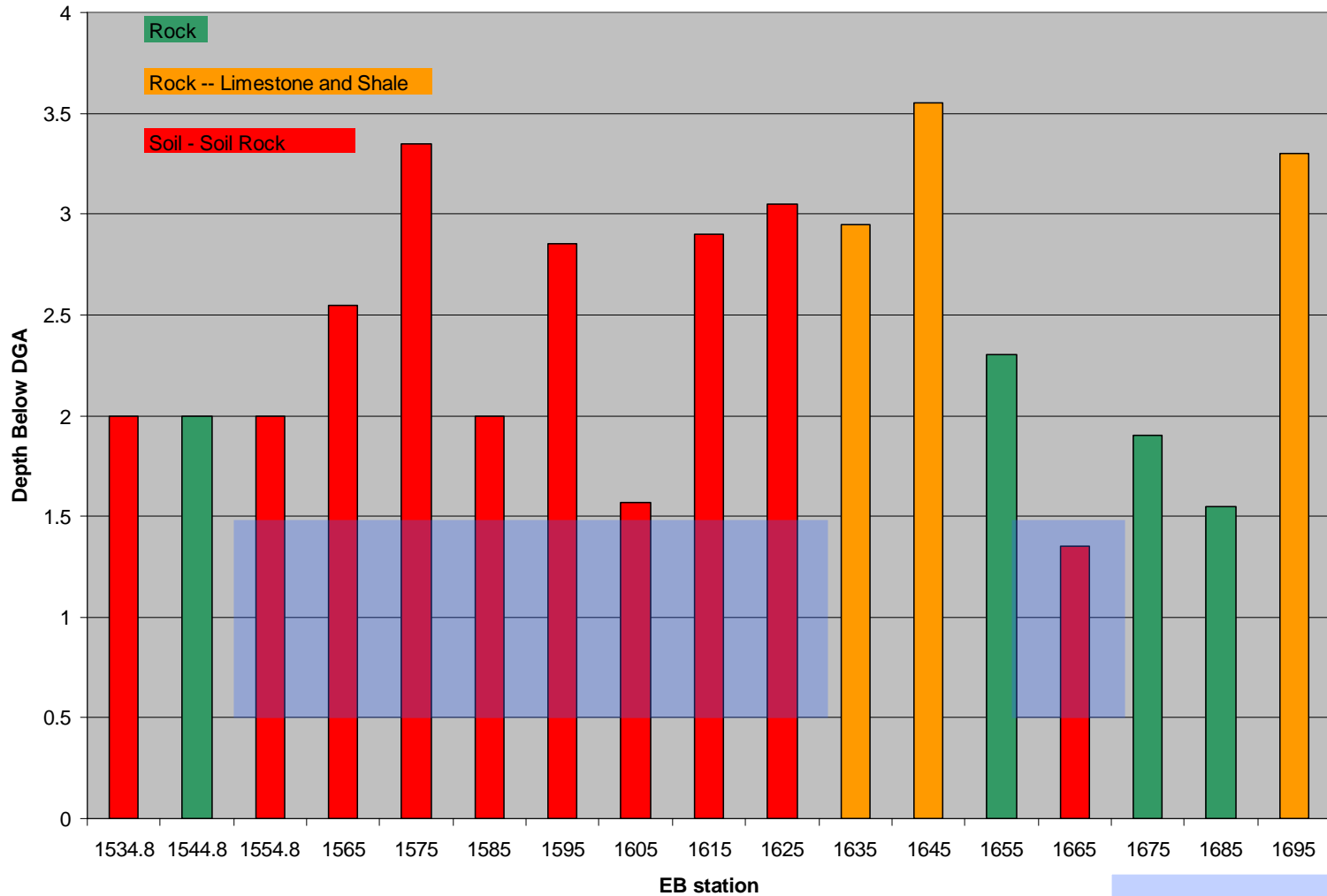
KYDOT compared information provided by
GPR to initial design proposals

Originally design firm proposed 7 different pavement rehabilitation designs

- \$6,710,373 remove right driving lane slab/replace, rework D.G.A. shoulders
- \$6,055,364 remove right driving lane slab/replace, rework D.G.A. shoulders
- \$8,260,344 remove right driving lane slab/replace, weekend work, rework D.G.A. shoulders
- \$13,196,072 remove all slabs/replace, install edge-drains, rework D.G.A. shoulders
- \$10,959,017 1" bond breaker—9" PCC overlay, rework D.G.A. shoulders
- ★ – \$8,258,970 break and seat existing pavement—10" asphalt overlay, rework D.G.A. shoulders
- \$17,476,275 remove all slabs/replace, install edge-drains, concrete shoulders

Additional GeoTech. work eastbound (boring log)

I-265 Eastbound

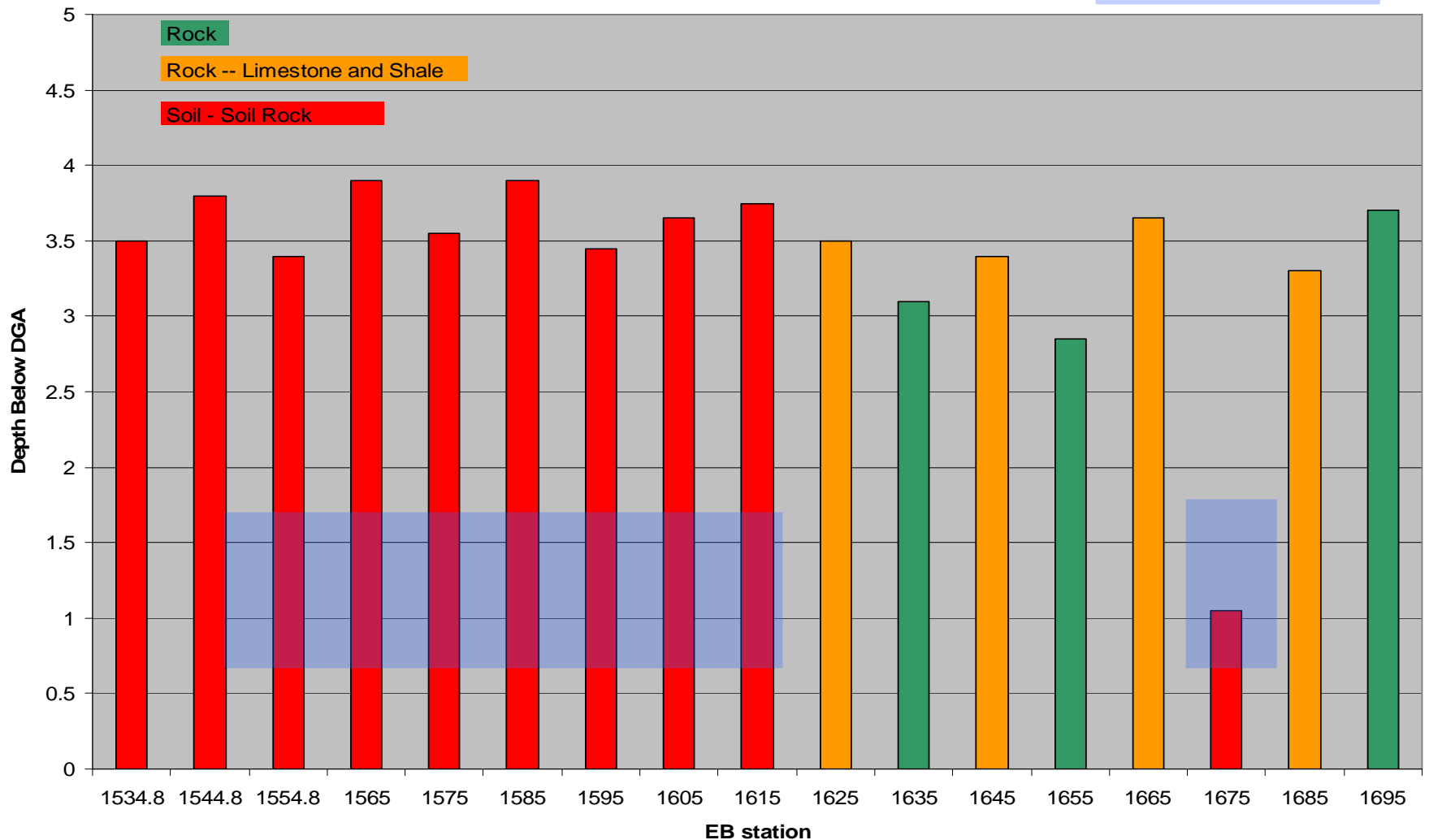


Clay layer (gpr)

Additional GeoTech. work westbound (boring log)

I-265 Westbound

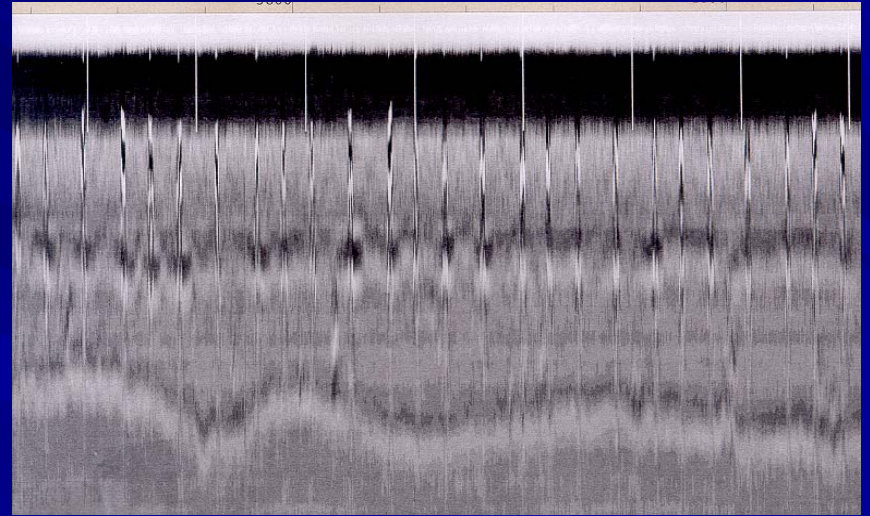
Clay layer (gpr)



A new pavement design was created

- Break and remove existing concrete pavement, excavate sub-grade (clay layer), replace with number 2 size stone, place new concrete pavement, and install edge-drains
- Project currently under construction approx. \$14 million

I-265 pavement structure after excavation



Why the pavement has settled

- Provided that traffic predominately travels in the right driving lane, over time the saturated clay beneath the DGA layer has compressed thus allowing for differential settlement of the right driving lane



Case Study: #2

Different strategies for collecting
concrete pavement distress information
for rehabilitation design purposes

Project: I-65 Central Kentucky

Case Study: #2

■ Project background

- 27-mile long project on I-65 in central, KY.
- 10 inch concrete pavement constructed in 1987
- Right driving lane has differentially settled approximately 1 to 2 inches from the shoulder and the left driving lanes in various areas
- Approach slabs have settled on three different bridges

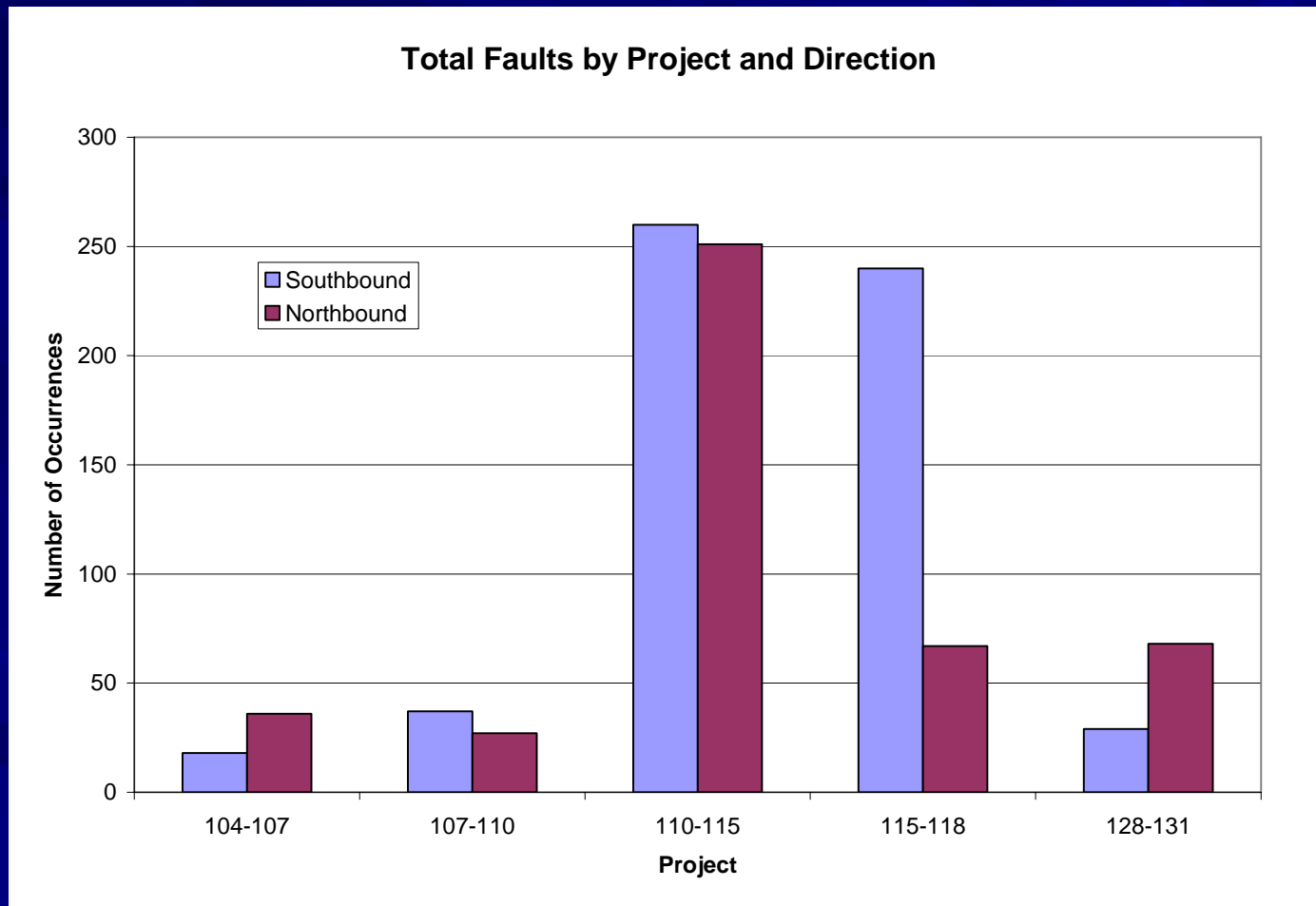
Pavement Evaluation

- Visual survey of cracking, settlement, and faulting
- FWD and CBR Analysis
- Assessment of tie assembly
- Degree of Moisture beneath the concrete slab
- Thickness of slab
- Subgrade thickness

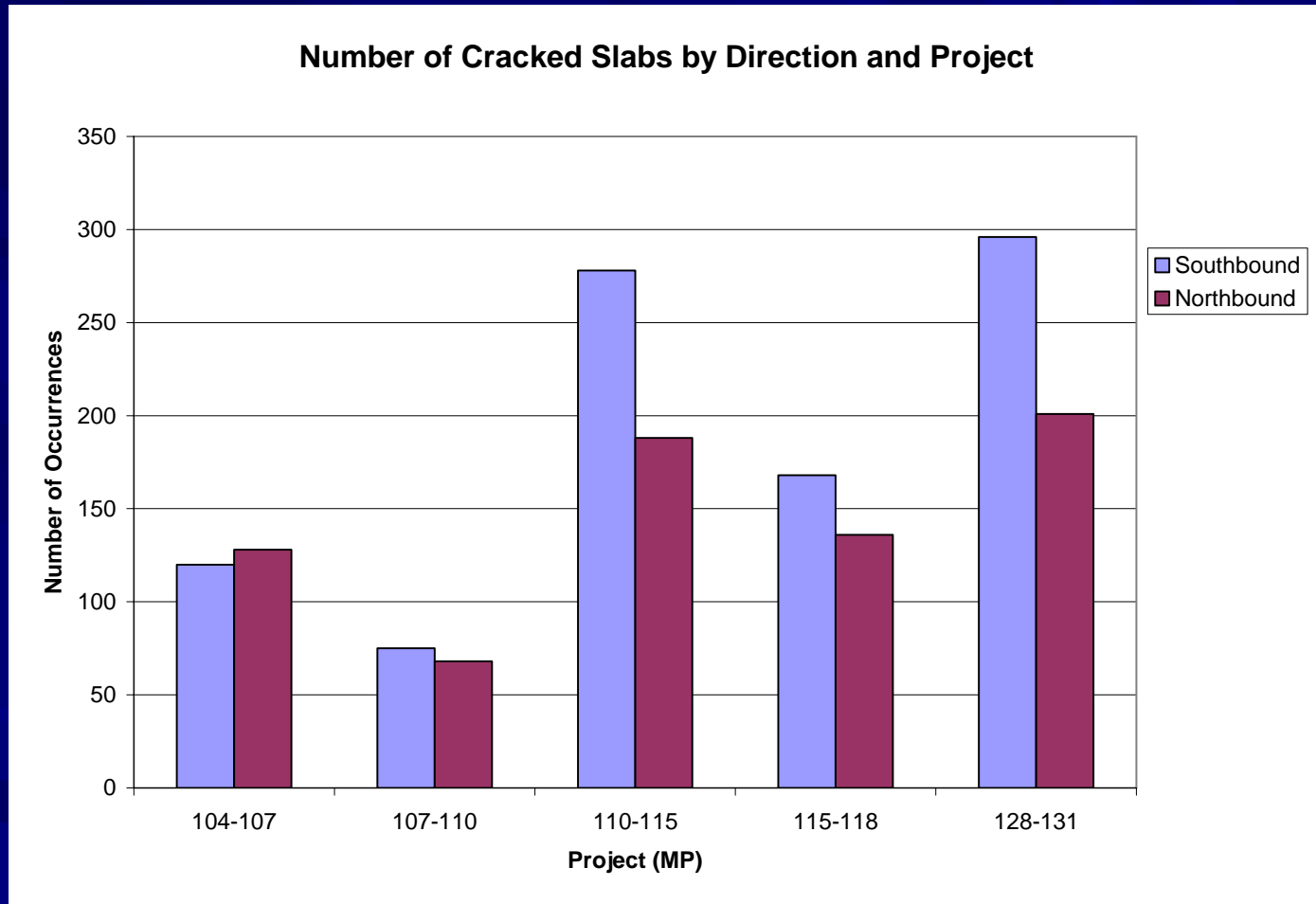
Visual survey of faulting, cracking, and settlement



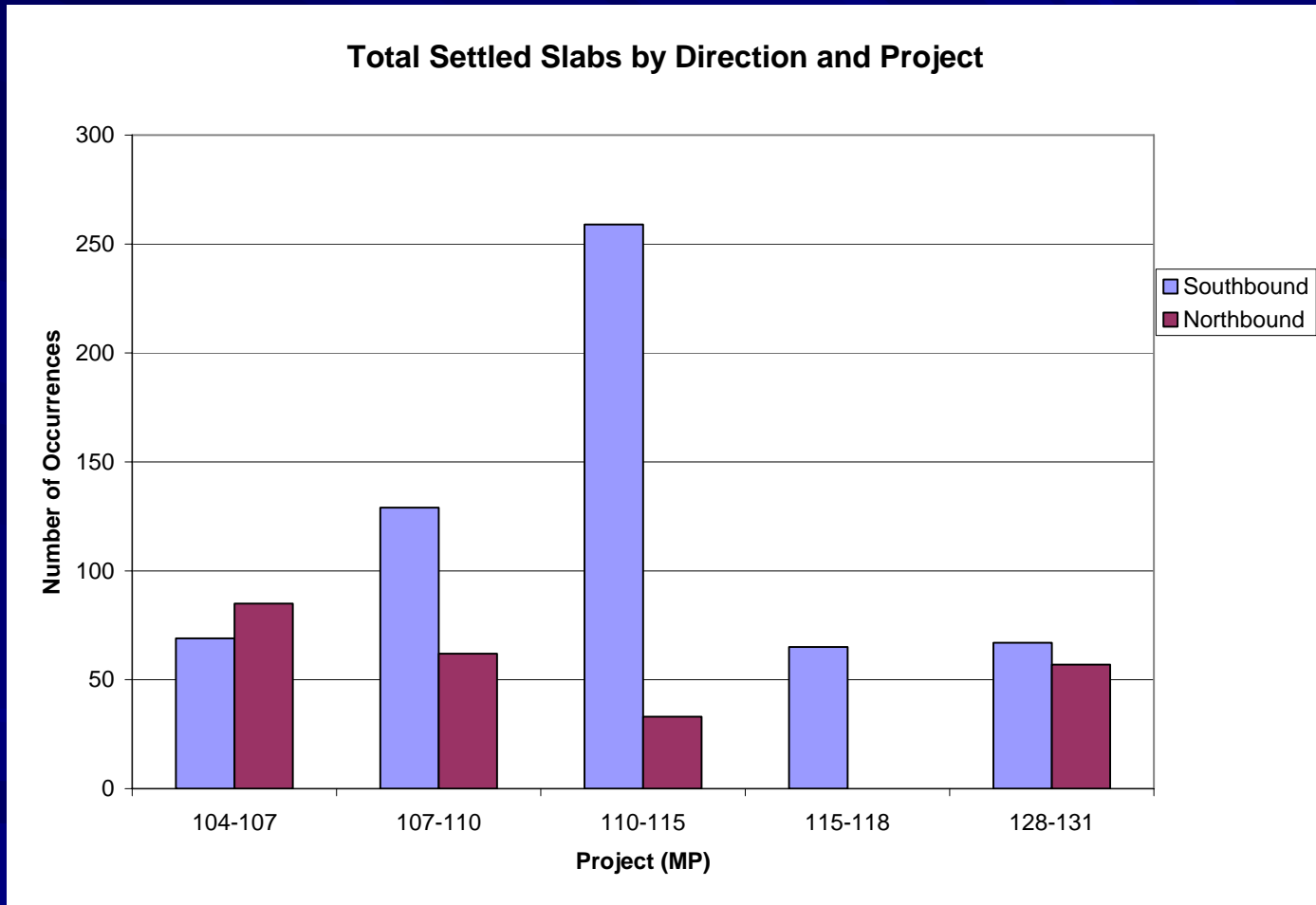
Number of Faulted Slabs per Project



Number of Cracked Slabs per Project



Number of Settled Slabs per Project

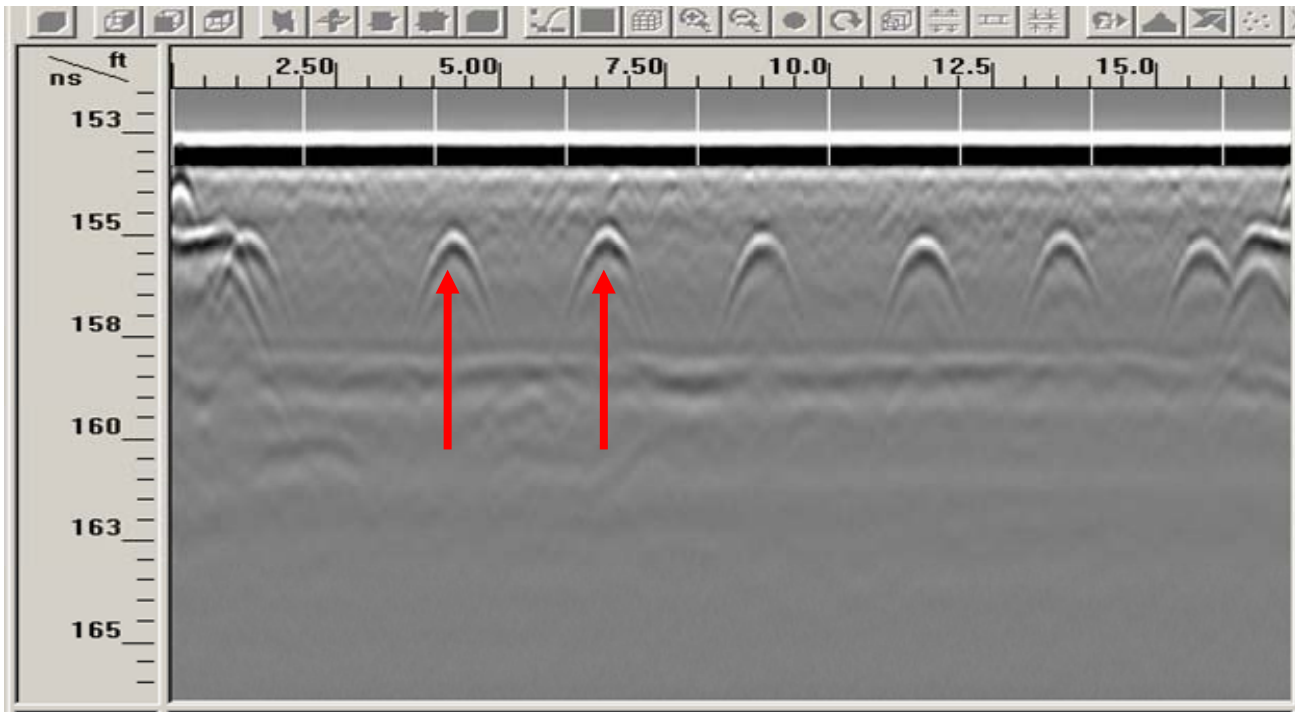


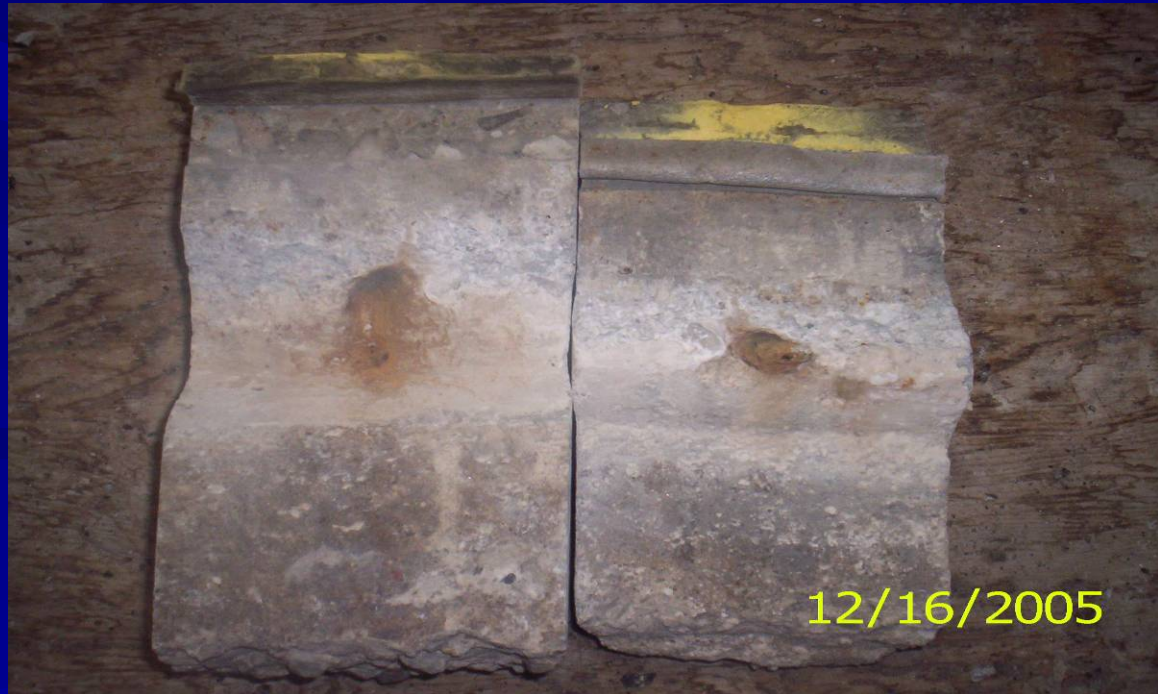
FWD and CBR Analysis

Station #	Load Transfer		Subgrade CBR	
	Inside Lane	Middle Lane	Inside Lane	Middle Lane
567+05	0.86	0.91	5.2	2.2
583+30	0.93	0.91	2.8	2.0
670+30	0.83	0.70	1.6	4.2

Assessment of tie assembly

Tie bar spacing between shoulder and right driving lane (30"):
Northbound station 765+00





Longitudinal Tie Assembly Keyed Joint



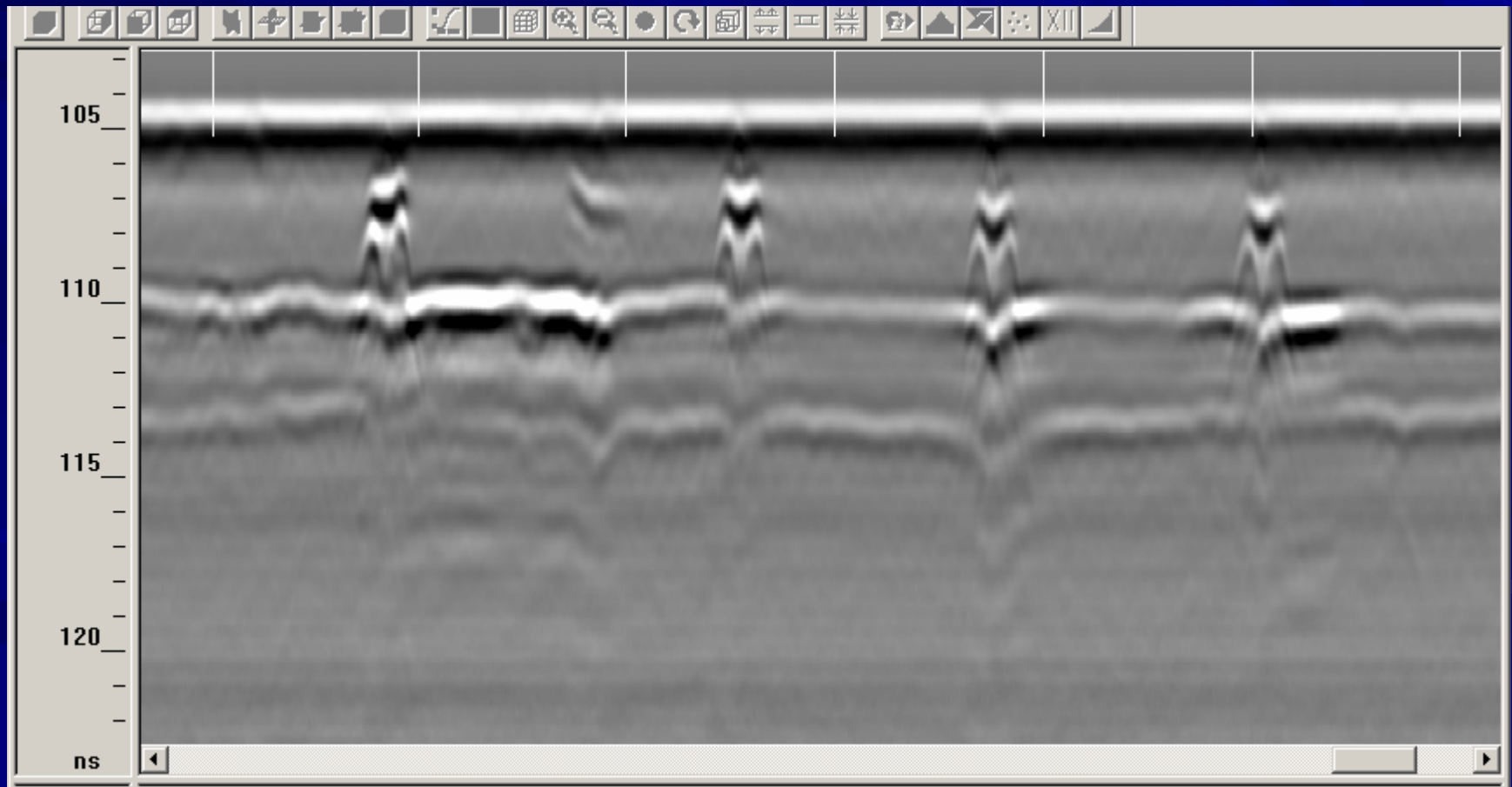
Degree of Moisture beneath the concrete slab



Water bleeding out of shoulder joint in patched area

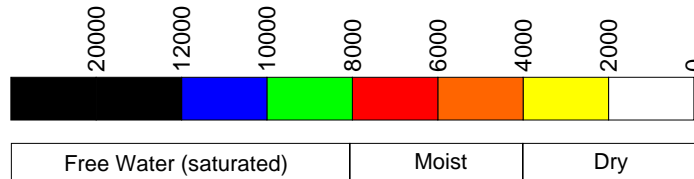


GPR data showing wet and dry sub-grade conditions beneath the concrete layer



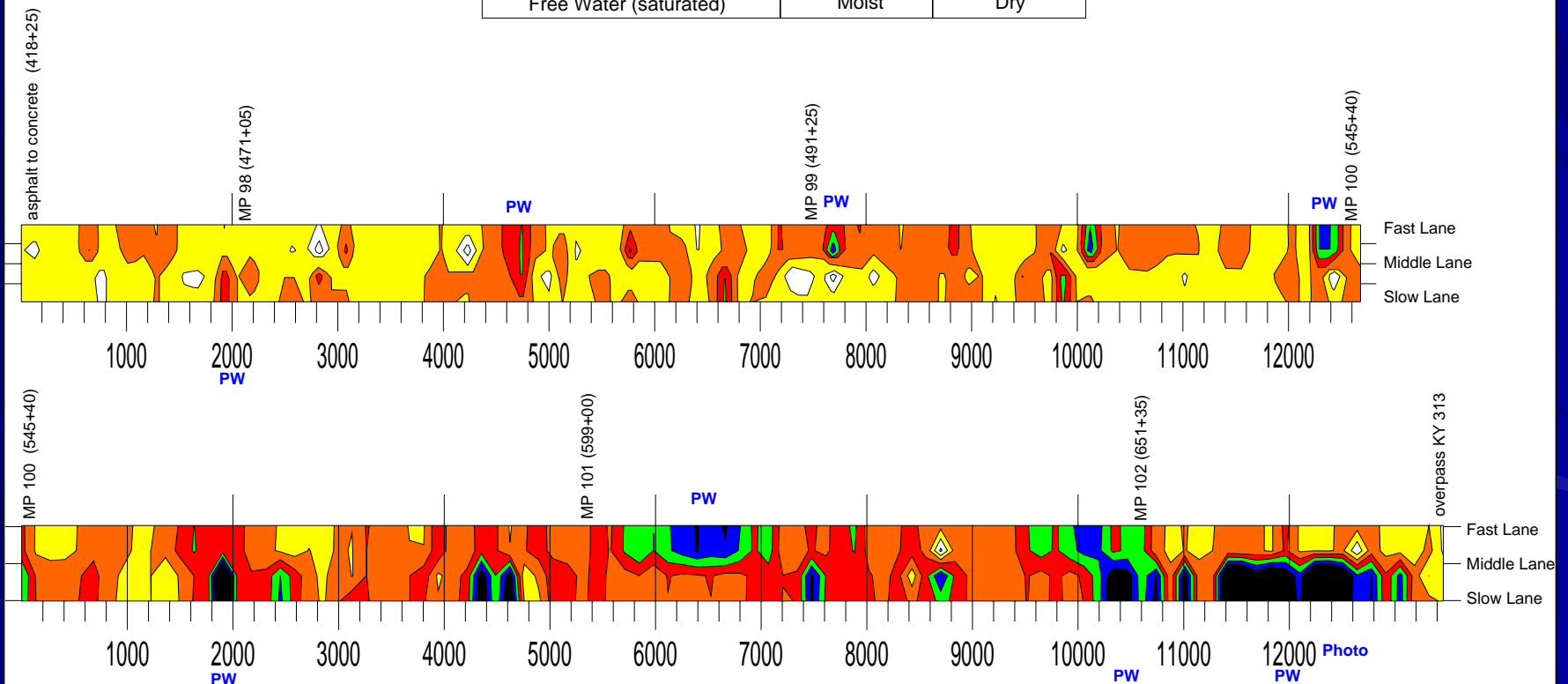
Northbound: degree of moisture beneath concrete

Northbound I-65: degree of moisture beneath concrete
 approx.milepoints 97.5 (asphalt to concrete) to 102.5 (KY 313 overpass)



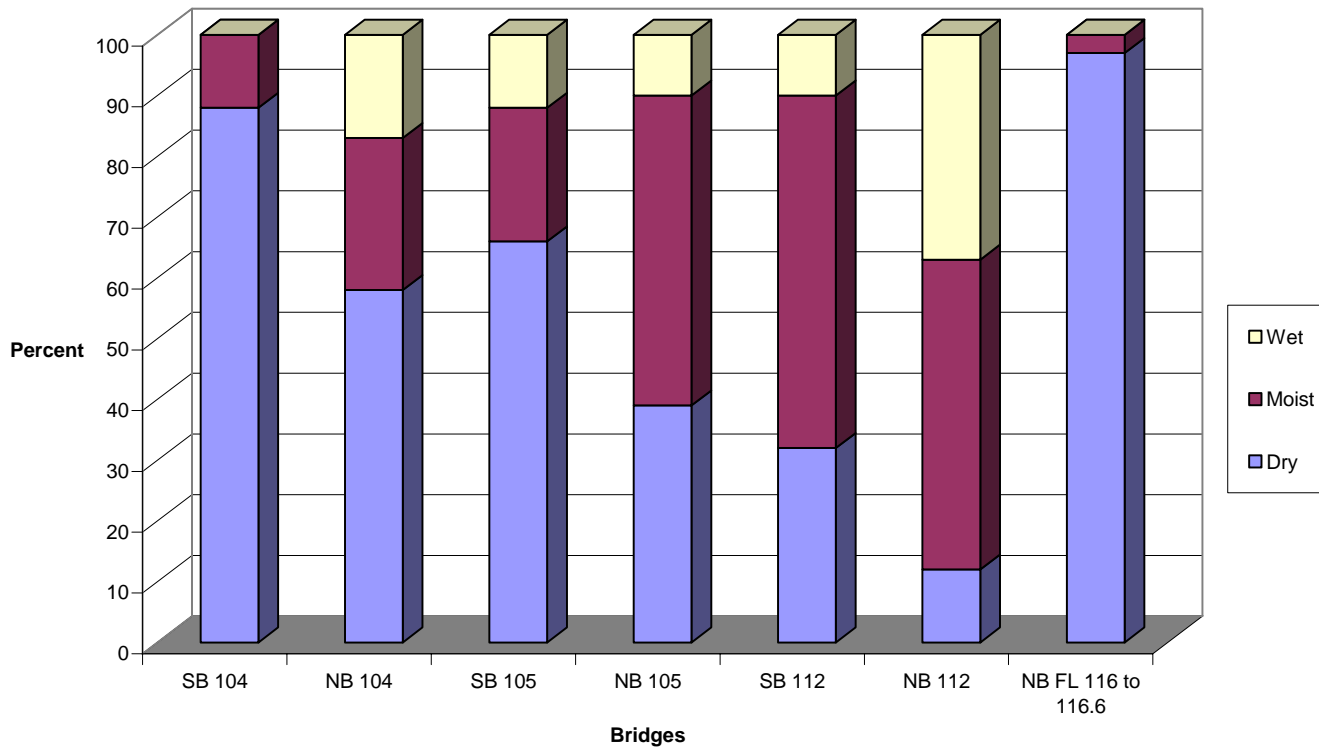
PW = Pumping water from long. joint onto shoulder

Photo = photo locations



Sub-grade moisture conditions for bridge-ends

Sub-grade conditions for bridge-ends and Northbound Fast Lane 116-116.6
(water content based on GPR amplitude less than 3500 dry, more than 6000 wet)



Pavement Conditions for bridge-ends

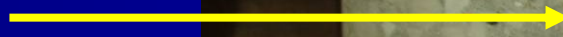


02/08/2004

Asphalt overlay



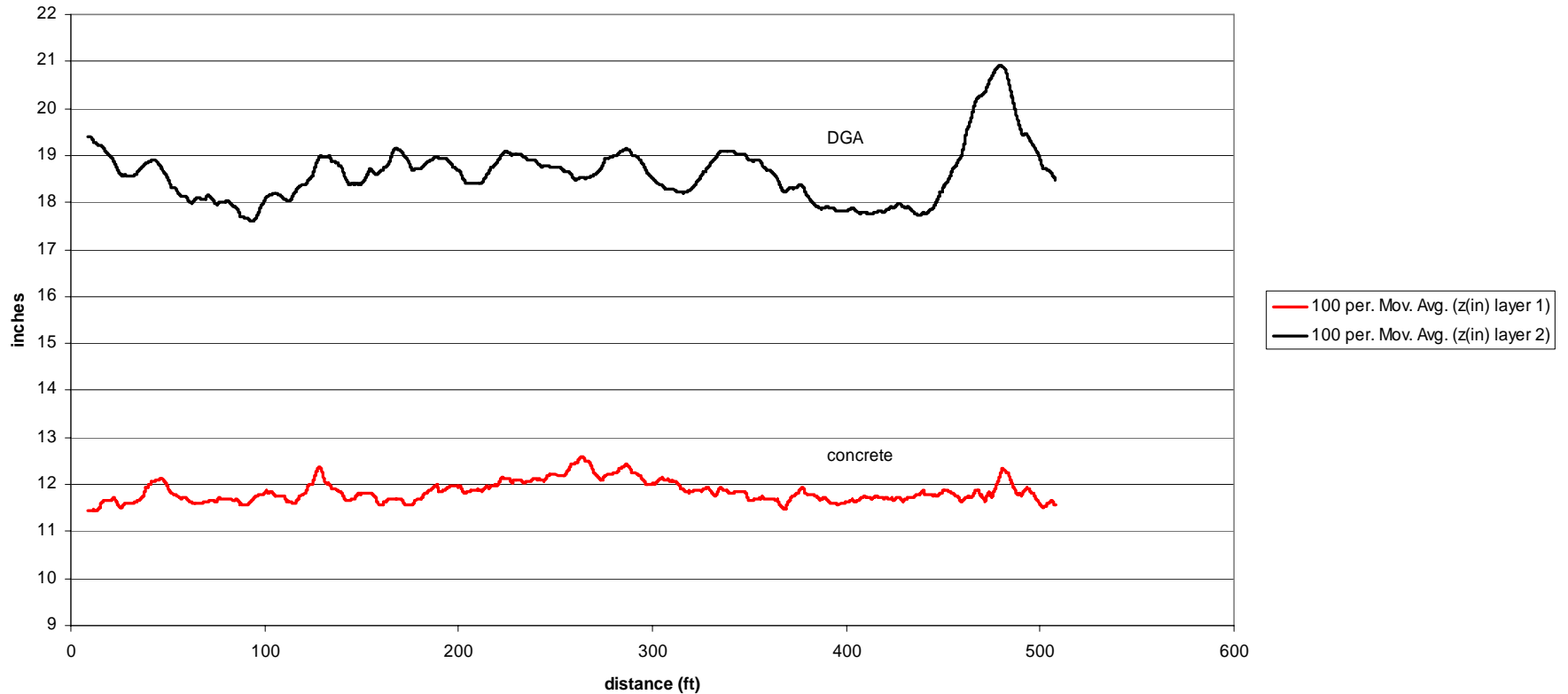
Foam jacking



Thickness of slab & sub-grade using GPR

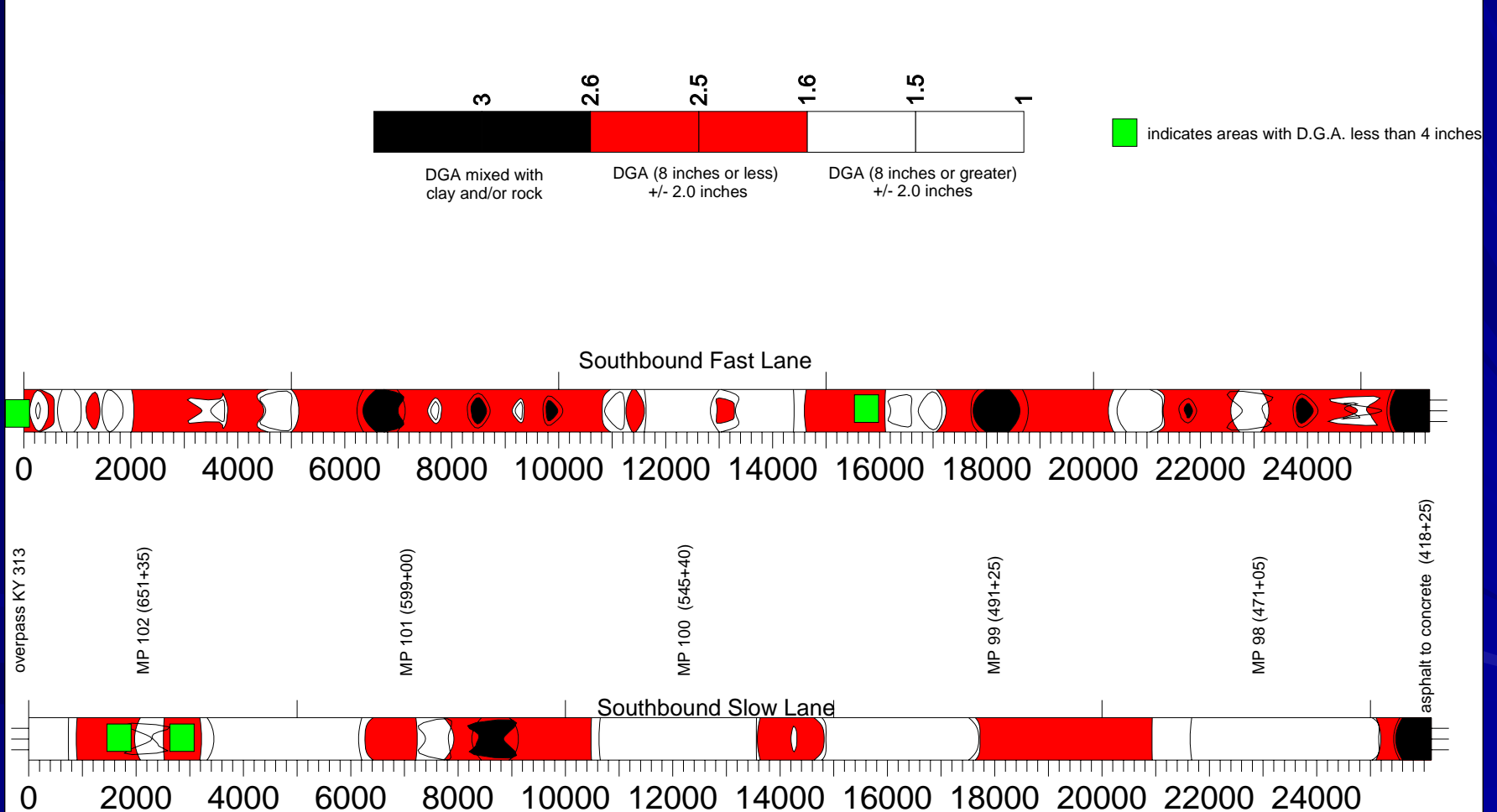
Pavement and sub-base thickness Northbound
station numbers 765+00 to 770+00
Section MP 104

North Bound Middle Lane



Sub-grade thickness and material type using GPR

I-65 Southbound: approx. subgrade thickness beneath concrete (+/- 2.0 inches)



Future Plans

To use some of the distress collection strategies discussed above to compare Kentucky's concrete pavements built in the 60's, that have an average age to rehabilitation of 26 years, to that of concrete pavements built in the 80's that have an average age to rehabilitation of 20 years

Conclusion

■ Case Study # 1:

- GPR prompted a more thorough geotechnical investigation on I-265 that ultimately lead to a more appropriate pavement design to fix the underlying problem causing the concrete pavement to settle

■ Case study # 2:

- Different strategies of collecting and reporting concrete pavement distress information for rehabilitation design purposes

Questions?

Thank You



For more information or a complete publication list, contact us at:

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