## ALTERNATIVE HMA CHARACTERIZATION FOR M-E PAVEMENT REHABILITATION DESIGN

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#### Outline

- Objective
- Background (Data Input Level for Rehabilitation Design)
- Approach
  - Field tests (Previous Study) + Laboratory Tests
- Results
- Findings and Conclusions
- Other Related Efforts



# **Acknowledgments**

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# **Project Objective**

- Determine the in-place HMA layer modulus for Mechanistic-Empirical Pavement Rehabilitation Design
- Verify the MEPDG proposed method for determining the field damaged HMA master curve
- Recommend how to improve it if found not applicable for VA pavements



# **Background:** Data Input Level 1

- **FWD DATA** 
  - Measure deflection
  - Backcalculate combined HMA modulus (E<sub>i</sub>)
- FIELD CORING
  - Volumetric properties
- UNDAMAGED MASTER CURVE

 $\log E^* = 3.750063 + 0.02932\rho_{200} - 0.001767(\rho_{200})^2 - 0.058097V_a - 0.802208\left(\frac{V_{beff}}{V_{beff}}\right)$ 

 $+\frac{3.871977 - 0.0021\rho_4 + 0.003958\rho_{38} - 0.000017(\rho_{38})^2 + 0.005470\rho_{34}}{1 + e^{-0.603313 - 0.313351\log(f) - 0.393532\log(\eta)}}$ 

$$\log |E^*| = \delta + \frac{\alpha}{1 + e^{\beta - \gamma \log f_r}}$$



#### Data Input Level 1 (Cont.)

ESTIMATE DAMAGE, dj:

$$\mathbf{d}_{j} = 1 - \frac{\mathbf{E}_{i}}{\mathbf{E}^{*}}$$

• DETERMINE  $\alpha$ '

$$\alpha' = (1 - d_j) * \alpha$$

← COMPUTE FIELD DAMAGED MODULUS  $\log |E^*| = \delta + \frac{\alpha}{1 + e^{\beta - \gamma \log f_r}}$ 





- FIELD CORING
  - Volumetric Properties

#### UNDAMAGED MASTER CURVE

 $\log E^* = 3.750063 + 0.02932\rho_{200} - 0.001767(\rho_{200})^2 - 0.058097V_a - 0.802208 \left(\frac{V_{beff}}{V_{beff}}\right)$ 

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$$\log |E^*| = \delta + \frac{\alpha}{1 + e^{\beta - \gamma \log f_r}}$$

## Data Input Level 2 (Cont.)

- A LABORATORY RESILIENT MODULUS, Mr
- ESTIMATE DAMAGE, dj:

$$d_j = 1 - \underbrace{M_r}_{E^*}$$

DETERMINE a'

$$\alpha' = (1 - d_j) * \alpha$$

COMPUTE FIELD DAMAGED MODULUS

$$\log |E^*| = \delta + \frac{\alpha}{1 + e^{\beta - \gamma \log f_r}}$$

Virginia



#### TYPICAL MIX PROPERTIES

#### UNDAMAGED MASTER CURVE

 $\log E^* = 3.750063 + 0.02932\rho_{200} - 0.001767(\rho_{200})^2 - 0.058097V_a - 0.802208$ 

$$\left( rac{V_{beff}}{V_{beff} + V_a} 
ight)$$

 $+\frac{3.871977 - 0.0021\rho_4 + 0.003958\rho_{38} - 0.000017(\rho_{38})^2 + 0.005470\rho_{34}}{1 + e^{-0.603313 - 0.313351\log(f) - 0.393532\log(\eta)}}$ 

$$\log |E^*| = \delta + \frac{\alpha}{1 + e^{\beta - \gamma \log f_r}}$$

## Data Input Level 3 (Cont.)

- **\* DISTRESS/CONDITION SURVEY** 
  - Pavement Rating: Excellent, Good, Fair, Poor, Very Poor
- ✤ ESTIMATE DAMAGE, d<sub>i</sub>:

Category	Damage (d <sub>i</sub> )
Excellent	0.00 - 0.20
Good	0.20 - 0.40
Fair	0.40 - 0.80
Poor	0.80 - 1.20
Very Poor	> 1.2



#### Data Input Level 3 (Cont.)

DETERMINE a'

$$\alpha' = (1 - d_j) * \alpha$$

**COMPUTE FIELD DAMAGED MODULUS** 

$$\log |E^*| = \delta + \frac{\alpha}{1 + e^{\beta - \gamma \log f_r}}$$



# **APPROACH**

- From previous study (different sites in VA)
  - ✓ FWD Testing → Backcalculated Moduli
  - ✓ Distress/Condition Survey
  - Coring + Laboratory Testing
    - Volumetric Analysis and Mr
- Calculate Estimated Undamaged E\*
- Calculate Damaged E\* for three input levels
- Measure E\* in the lab
- Compare





## **Selected Sites**





# Selected Sites (Cont.)

Site	County	Route	Directio n	Milepost*	<b>Pavement</b> Type	Pav. Age/ Surf.
1	Amherst	US-29	South	7.80-7.30	Flexible	34 / 11
3	Louisa	I-64	West	9.91-9.41	Flexible	34 / 9
6	York	I-64	West	2.62-2.12	Flexible	25 / 7
12	Greensville	I-295	North	5.50-6.00	Comp. JPCP (rehab)	14 / 6
14	Russell	I-19	North	8.68-9.18	Flexible	6/6
15	Rockbridge	I-81	South	22.92-22.42	Flexible	37 / 17
16	Frederick	I-81	North	21.31-21.87	Flexible	39 / 13
17	Washington	I-81	South	12.50-12,00	Flexible	42 / 11
18	Washington	I-81	South	1.50-1.00	Flexible	5/3







## Field Data Collection





Inference, Panama City, FL

# **FWD** Testing

- Test pattern dependent on pavement structure
- Basin Testing
  - 4 load levels and 3 drops per load level
  - 50 foot intervals in OWP (AC surface)





## FWD Analysis: Flexible Site 16



# **Coring and Boring**

- A Retrieve pavement materials samples for laboratory testing
- Number of tests varied based on pavement type
- Controlling factor was amount of lane closure time





#### VT Digital Camera System





2006 Southeastern Pavement Management & Design Conference

# **Pavement Condition**

Site	NDR	LDR	CCI
1	74.5	96.7	75
3	68.6	95.8	69
6	80.5	96.3	81
12	97.2	95.8	96
14	99.9	99.9	100
15	92.3	89.7	90
16	91.6	99.8	92
17	92	94.2	92
18	100	100	100



## Other Measurements...





Virgi<u>nia</u>

Tech



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# Laboratory Tests

Specimen preparation

Core 6" cores to 4"

Cut the ends



Dynamic Modulus
 5 Temperatures: 10°F, 40°F, 70°F, 100°F, 130°F
 6 Frequencies: 0.1Hz, 0.5Hz, 1Hz, 5Hz, 10Hz, 25Hz



## **Dynamic Modulus**









esign Conference, Panama City, FL



# Results: Site 01 (Core S01C4)

Volumetric Analysis
 Ignition
 Gradation

Undamaged E\*
 Witczak equation



# **Estimated Undamaged Master Curve**



Backcalculated Modulus:
 ✓ E (avg.) = 462ksi, Avg. Temp. = 74.2°F

#### Undamaged E\* @ 5Hz, 74.2°F = 672ksi

$$d_{j} = 1 - \frac{462}{672} = 0.31$$

$$\alpha' = 0.69 * 3.86 = 2.66$$



- Resilient Modulus
  - ✓ Mr (Wearing Surface) = 746ksi
  - ✓ Mr (Base) = 433ksi

$$\mathbf{M}_{\rm rc} = \left(\frac{\mathbf{h}_1 \sqrt[3]{\mathbf{M}_{\rm rWS}} + \mathbf{h}_2 \sqrt[3]{\mathbf{M}_{\rm rBM}}}{\mathbf{h}_1 + \mathbf{h}_2}\right)^3$$

$$M_{rc} = 569$$

✓ Undamaged E\* @ 1.6Hz, 77°F = 468ksi  $d_j = 1 - \frac{569}{468} = -0.22$  $\alpha' = 1.22 * 3.86 = 4.71$ 



#### 

#### Arrow Rating = Good

 $d_{j} = 0.3$ 

$$\alpha' = 0.7 * 3.86 = 2.7$$





**Reduced Frequency (Hz)** 







Design Conference, Panama City, FL

#### S03C7



**Reduced Frequency (Hz)** 



# **SITE 06**



#### **S06C1**



**Reduced Frequency (Hz)** 







ment & Design Conference, Panama City, FL

#### S12C2



**Reduced Frequency (Hz)** 



- Volumetric properties from the same site were different from core to core, which resulted in different measured dynamic modulus
- Sigmoidal function provides a very good fit to the dynamic modulus master curve



# Findings (Cont.)

- Laboratory-measured resilient modulus test does not give a good indication of the combined behavior of thick HMA layers
- Using the average values for the backcalculated FWD may not provide the best estimates for the damage factor



# Findings (Cont.)

- The Witczak prediction equation gives reasonable values for the dynamic modulus
  - Same order of magnitude as the measured ones
  - In most cases: 0.3 < E<sub>predicted</sub>/E<sub>measured</sub> < 1.8</pre>



# Findings (Cont.)

 The use of level 3 data on previously overlaid pavements may be misleading because the surface condition does not reflect the overall condition of the entire HMA layer



# CONCLUSIONS

- Level 2, as currently used in the proposed M-E Pavement Design Guide, provides unreasonable values for the damaged dynamic modulus master curves
  - Linked to the method used to measure the Resilient Modulus?



 Level 1 data is necessary to obtain reliable estimates of the properties of the existing HMA layers.

 FWD testing appears to be the only reliable procedure that can measure the overall condition of the entire HMA layer



# **RECOMMENDATIONS**

- VDOT should not use level 2 type of input for rehabilitation when the proposed ME guide is implemented
- Perform sensitivity analysis on the effect of the existing HMA layer dynamic modulus values on the overlay design



# **Related Projects**

- Catalog of HMA properties for MEPDG
  - 3 surface, 4 intermediate & 4 base mixes
  - ✓ Dynamic Modulus
  - Creep
  - Resilient Modulus
  - ✓ IDT

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Distress Model Calibration (pending approval)





